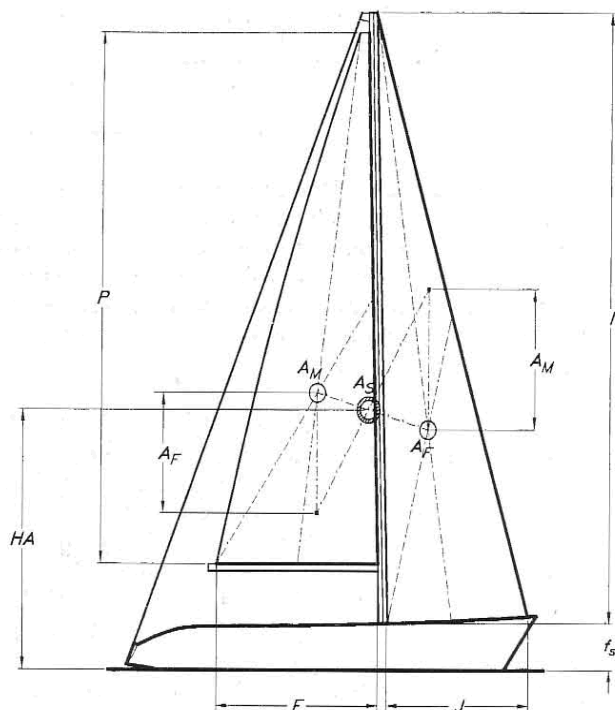


RIG CONSTRUCTION

Definitions and righting moments



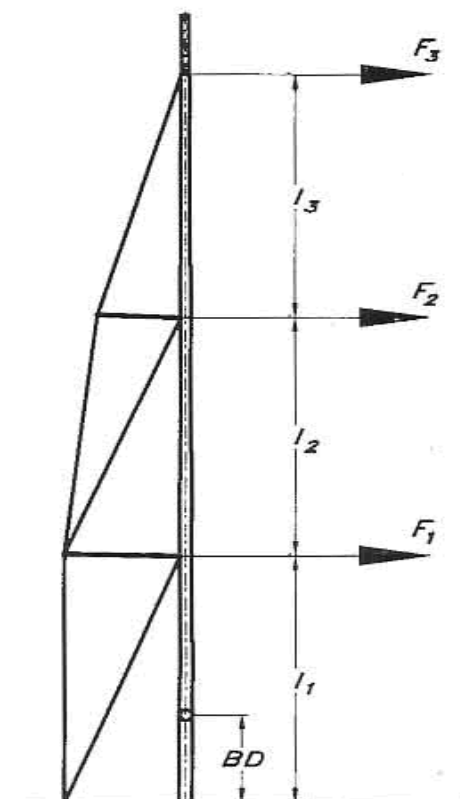
Dimentionig righting moment

G -empty weight of boat (kg)	5749 kg
Δ - full load weight of boat (kg)	6700 kg
g - ballast weight (kg)	2500 kg
Loa - length overal (m)	11,97 m
Boa (B) - beam overall	4 m
As - sail area (m2)	90 m2
RM30 - righting moment at 30deg. heel empty boat (Nm)	40180 Nm
RM1 - righting moment at 1deg. heel empty boat (Nm)	
n - number of person onboard	6
Fs - freeboard at mast (m)	1,3 m
σ RM - additional moment from crew to windward (Nm)	Nm
HA - heelling arm (m)	7,82 m
$A_m = E \cdot P / 2$	38 m2
$A_f = I \cdot J / 2$	42 m2
$A_s = A_m + A_f$	90 m2
$\sigma RM = 75 \cdot n \cdot (3,4B - 4,9Fs)$	3254 Nm
RM = RM30 * Δ / G + σ RM	50080 Nm

Forces from shrouds

Case 1. Dimentioning transverce force T1 (only foresail)	$T1 = RM / a1 =$	2823 N
a1- distance from WL to appermost shroud = 17,74m		17,74 m
Case 2. Dimentioning transverce force T2 (only deeply reefed main)	$T2 = RM / a2$	8634 N
a2- distance from WL to geometr. centre of reefed mainsail =		5,8 m
Thu - force acting on upper shrouds	$Thu = Thead \cdot d1 / d1 + d2$	3455 N
Thl - force acting on lower shrouds	$Thl = Thead \cdot d2 / d1 + d2$	997,4 N
Tbu - force acting on lower shrouds from the boom	$Tbu = Tboom \cdot BD / L1$	750,3 N
Thead - force acting on the mainsail head	$Thead = 0,4 \cdot T2$	3454 N
Tboom - force acting on the boom	$Tboom = 0,33 \cdot T2$	2849 N
d1- distance from mainsail head to lower shrouds		4,23 m
d2- distance from mainsail head to upper shrouds		1,22 m
BD - distance from deck to boom		1,295 m
L1 - Distance from deck to first spreaders		4,918 m
L2 - Distance from first spreaders to second spreaders		5,44 m
L3 - Distance from second spreaders to upper shrouds		5,51 m
$\beta 1 =$	13 deg. 0,23 rad.	
$\beta 2 =$	13 deg. 0,23 rad.	
$\beta 3 =$	11 deg. 0,19 rad.	
$\gamma 1 =$	0,3 deg. 0,01 rad.	
$\gamma 2 =$	2,6 deg. 0,05 rad.	
I=	16,33 m	
P=	15 m	
E=	5,19 m	

Dimentioning Forces for Shroud and Shroud load



F-2

Dimentioning Forces F1, F2, F3						
Type of rig	Case 2(only r/ main)			Case1(only staysail)		
	F1	F2	F3	F1	F2	F3
F0	Thu+Tb	0	0	T1	0	0
M-1/F-1	Thl+Tbu	Thu	0	0	T1	0
M-2/F-2 (1*)	Tbu	Thl	0	0	0	T1
M-2/F-2 (2*)	Thl+Tbu	Thu	0	0	0	T1
Meanings	1747,6	3455	0	0	0	2823

1* - if $BD+0,6P>L1+L2$

2* - if $BD+0,6P<L1+L2$

$BD+0,6P = 10,271$

$L1+L2= 10,358$

$L1 = 4,918 \text{ m}$

$L2 = 5,44 \text{ m}$

$L3 = 5,51 \text{ m}$

$BD = 1,295 \text{ m}$

Shroud Tension (D#, V#)

$D3 = F3/\sin\beta_3 = 14795 \text{ N}$

$V2 = F3/(\cos\gamma_2*\tg\beta_3)= 14538 \text{ N}$

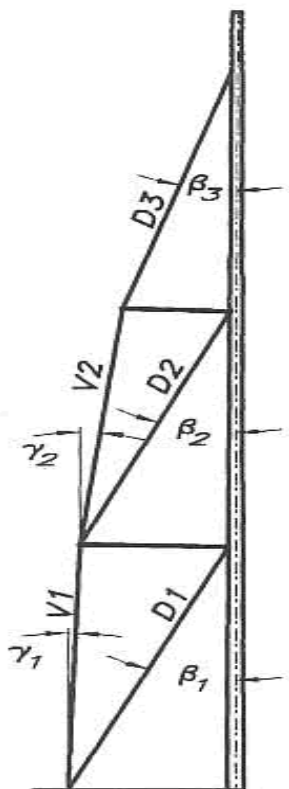
$C2 = F3 - V2*\sin\gamma_2 = 2164 \text{ N}$

$D2 = (F2+C2)/\sin\beta_2 = 24977 \text{ N}$

$V1 = (F2+C2)/(\cos\gamma_1*\tg\beta_2) + V2*\cos\gamma_1/\cos\gamma_2= 38890 \text{ N}$

$C1 = F2+C2+ V2*\sin\gamma_2-V1*\sin\gamma_1 = 6074 \text{ N}$

$D1 = (F1+C1)/\sin\beta_1 = 28751 \text{ N}$



F-2

Dimentioning Loads (P#)

$PD1 = 2,8*D1(\text{single lower shrouds}) = 80502 \text{ N}$

$PD1 = 2,5*D1(\text{double lower shrouds}) = 71877 \text{ N}$

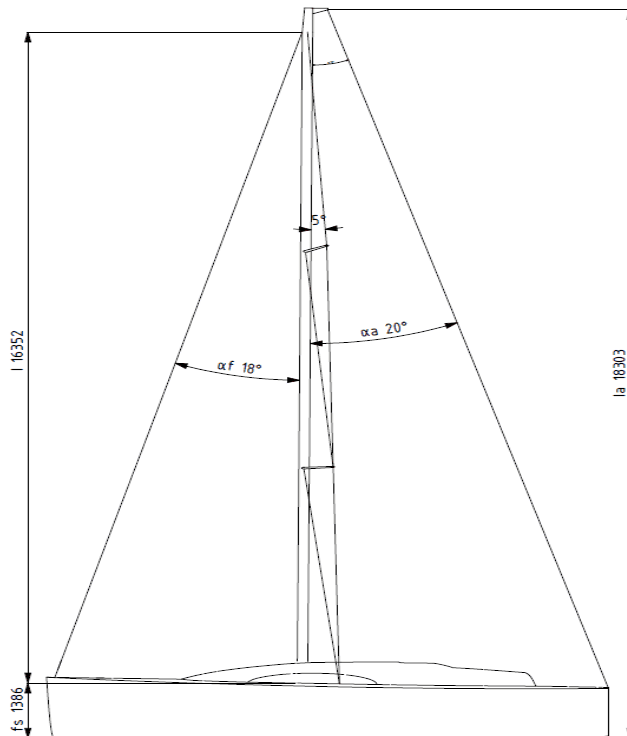
$PD2 = 2,3*D2 = 57446 \text{ N}$

$PD3 = 3,0*D3 = 44400 \text{ N}$

$PV1 = 3,2*V1 = 124447 \text{ N}$

$PV2 = 3,0*V2 = 43614 \text{ N}$

Forces on the stays



The foremost sail carrying forestay shall have a breaking strength (Pfo) of at least:

$$Pfo = 15 \cdot RM / (I + Fs) = 42609 \text{ N}$$

The inner forestay shall have a breaking strength (Pfi) of at least:

$$Pfi = 12 \cdot RM / (I + Fs) = 36802 \text{ N}$$

The aft stay shall have a breaking strength (Pfi) of at least:

$$Pa = Pfo \cdot \sin \alpha_f / \sin \alpha_a = 38498 \text{ N} \quad (\text{masthead rig})$$

$$Pa = 2,8 \cdot RM / (Ia \cdot \sin \alpha_a) = 22355 \text{ N} \quad (\text{fractional rig})$$

$$\alpha_f = 18 \text{ deg.} \quad 0,314 \text{ rad.}$$

$$\alpha_a = 20 \text{ deg.} \quad 0,349 \text{ rad.}$$

$$Ia = 18,34 \text{ m}$$

Transverse mast dimensioning

Required transverse moment of inertia (Ix) for the mast:

$$Ix = k1 \cdot m \cdot PT \cdot L(n)^2 = 4689230 \text{ mm}^4$$

$$PT = 1,5 \cdot RM / b = 55235 \text{ N}$$

$$k1 - \text{panel factor (from table below)} = 3,51$$

$$m - \text{for aluminium} = 1$$

L(n) - actual panel length

$$k3 = 1,35 \text{ for deck stepped mast} \quad 1,35$$

$$k3 = 1,0 \text{ for deck stepped mast} \quad 1$$

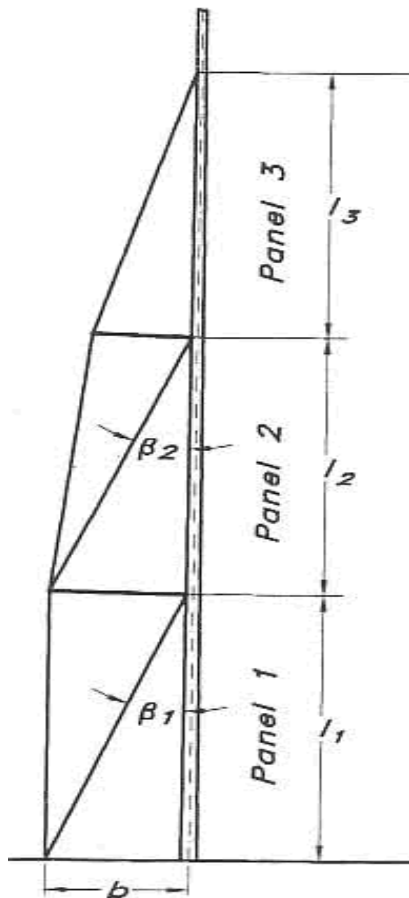
$$b = 1,36 \text{ m}$$

When calculating Ix for panel 2 PT can be decreased by:

$$D1 \cdot \cos \beta_1$$

When calculating Ix for panel 3 PT can be decreased by:

$$D1 \cdot \cos \beta_1 + D2 \cdot \cos \beta_2$$



Type of rig	Panel Factor k1		
	Panel1	Panel 2 & 3	
F-0	2,4*k3		
F-0 short spr.	1,6*k3		
M-1	2,3*k3	3,5	3,5
F-1	2,4*k3	3,35	3,35
M-2	2,7*k3	3,8	3,8
F-2	2,6*k3	3,6	3,6

Panel 1

$$Ix = k1 \cdot m \cdot PT \cdot L1^2 = 4689230 \text{ mm}^4$$

Panel 2

$$Ix = k1 \cdot m \cdot (PT - D1 \cdot \cos \beta_1) \cdot L2^2 = 2827586 \text{ mm}^4$$

Panel 3

$$Ix = k1 \cdot m \cdot (PT - (D1 \cdot \cos \beta_1 + D2 \cdot \cos \beta_2)) \cdot L3^2 = 307425 \text{ mm}^4$$

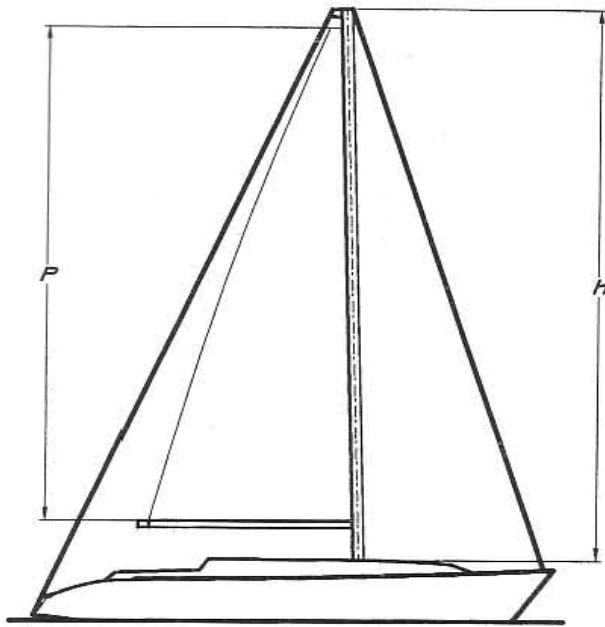
M-2/F-2

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ROOTS Rig Construction

sheet

Longitudinal mast dimentioning



Regired longitudinal moment of inertia for mast (I_y):

$$I_y = k_2 \cdot k_3 \cdot m \cdot P T \cdot h^2 = 15024325 \text{ mm}^4$$

$$P T = 1,5 \cdot R M / b = 55235 \text{ N}$$

$$k_2 - \text{staying factor (from table below)} = 0,8$$

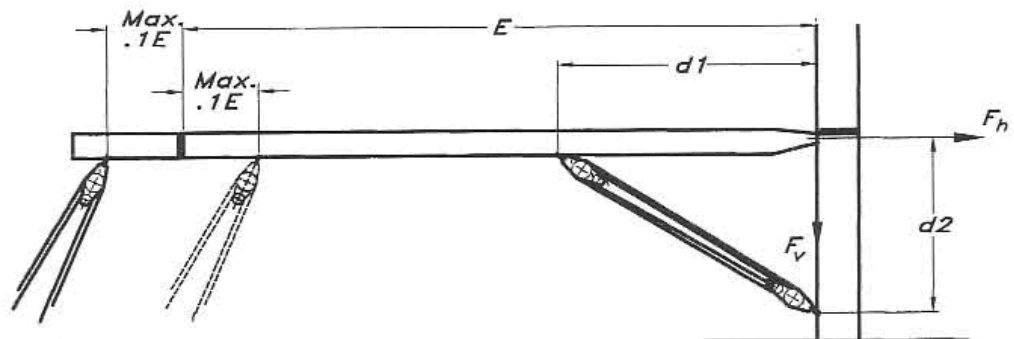
$$m = 1 \text{ for aluminium} = 1$$

$$k_3 = 1,35 \text{ for deck stepped mast} = 1,35$$

$$h - \text{height above deck or superstructure to the highest sail carrying forestay} = 15,87 \text{ m}$$

Type of staying	Staying Factor k_2				
	F-0	M-1	F-1	M-2	F-2
Double lowers		0,85	0,8	0,9	0,85
Single lowers		0,8	0,75	0,85	0,8
Runners & i.1			0,85		0,8
Runners & c.s		1	0,95	0,95	0,9
Swept spreadr.			1		0,95
Short spreadrs	1,05				
No spreaders	2				

Boom requirement



The gooseneck shall be able to withstand a vertical and horizontal force of:

$$F_v = 0,5 \cdot R M \cdot E / (H A \cdot d_1) = 10387 \text{ N}$$

$$F_h = 0,5 \cdot R M \cdot E / (H A \cdot d_2) = 13622 \text{ N}$$

$$H A - \text{distance from WL to centre of effort of sails} = 7,82 \text{ m}$$

$$E = 5,19 \text{ m}$$

$$d_1 = 1,6 \text{ m}$$

$$d_2 = 1,22 \text{ m}$$

$$\sigma_{0,2} - \text{yield strength for spreaders (N/mm}^2\text{)} = 210 \text{ N/mm}^2$$

Regired vertical Section Modulus for the boom is:

$$S M = 600 \cdot R M \cdot (E - d_1) / (\sigma_{0,2} \cdot H A) = 65688 \text{ mm}^3$$

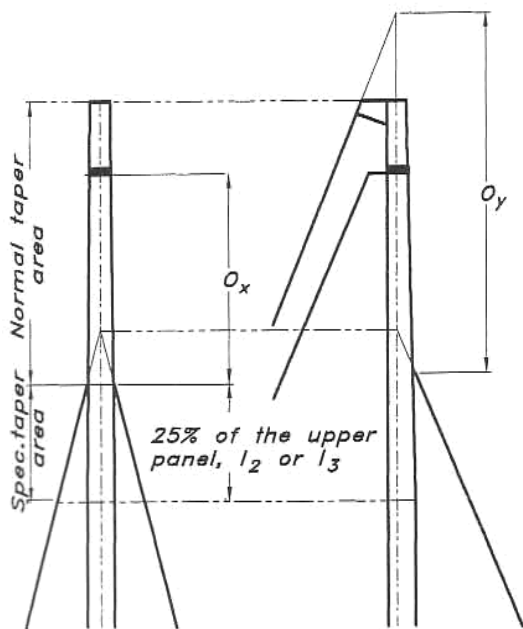
$$\text{The horizontal Section Modulus is to be at least 50\% of the vertical} = 32844 \text{ mm}^3$$

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ROOTS Rig Construction

sheet

Fractional mast top



The Section modulus for the mast top in fractional rigs, shall at the intersection of the forestay of upper shrouds and mast be:

$SM_x = 8 \cdot RM \cdot O_x / P =$ mm³

$SM_y = 2100 \cdot RM \cdot O_y / (\sigma_{0,2} \cdot (O_y + h)) =$

$\sigma_{0,2}$ - yield strength of mast = 210 N/mm²

At the top of the mast the required Section Modulus is half of the above calculated

Tapering of SM_x and SM_y might be done down to a level of 25% of the panel length below the upper shrouds/mast intersection and upper spreaders

If O_x is less than 6% of the mast length (h) then the rig is considered to be a masthead rig.

O_y = m
O_x = m
h = 15,87 m

Holes in the mast

Reduction of Section modulus of mast

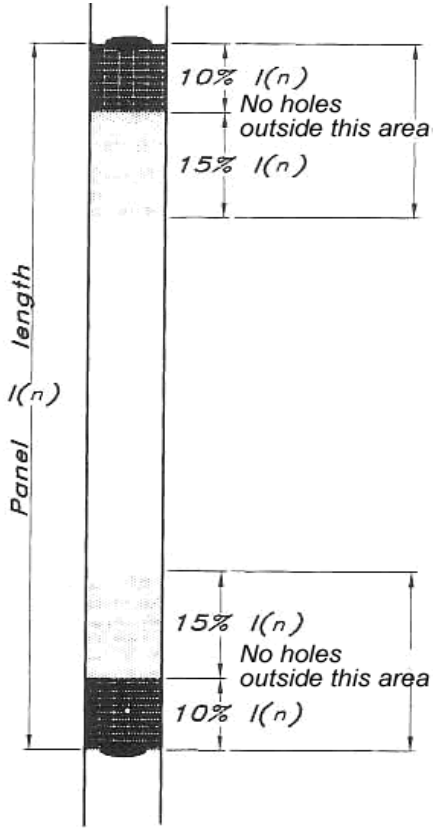
In the 10% L(n) area the Section Modulus is allowed to be decreased by 50%

In the 15% L(n) area the Section Modulus is allowed to be decreased by 30%

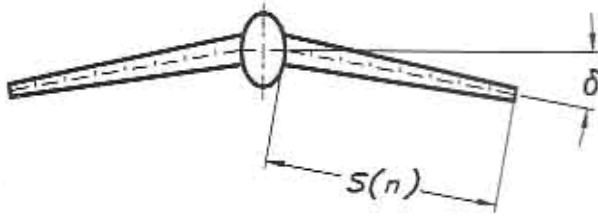
The Modulus reduction is (I_m):

$I_m = b \cdot t \cdot r$

b - width of hole = mm
t - wall thickness = mm
r - distance to neutral axis = mm



Spreader requirement



The moment of inertia of the spreader at half span is to be:

$$I = 0,8 \cdot C(n) \cdot S(n)^2 / (E \cdot \cos \sigma) = 127213 \text{ mm}^4$$

E - modulus of elasticity of spreader = 69340

S(n) - length of spreader 1260 mm

sigma - horizontal angle of spreader = 29 deg. 0,51

C(n) - transverse component of shroud force = 6074 N

Close to the mast spreader shall have a Section Modulus of

$$SM = k \cdot S(n) \cdot V(n) \cdot \cos \sigma = 32653 \text{ mm}^3$$

$$k = 0,16 / \sigma_{0,2} = 8E-04$$

Vn = V1 for lower spreaders, D3 - for upper spreaders

38890 N/mm²

sigma_{0,2} - yield strength for spreaders (N/mm²) = 210 N/mm²

The spreaders attachment shall be able to withstand a moment of:

$$Ms = 0,16 \cdot S(n) \cdot \cos \sigma = 176,3 \text{ Nmm}$$

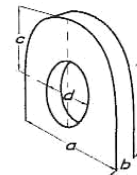
Enter in the table below with all values to pick the relevant shrouds, stays and rig components.

Typical properties for aluminium extrusions

Mast	Main Dim. (mm)	I _y (cm ⁴)	I _x (cm ⁴)	Wall Thkn. (mm)	Weight Kg/m	SM _y (cm ³)	SM _x (cm ³)
Oval Sect.	122/85	165	75	2.45	2.43	23.6	17.6
	130/93	215	100	2.50	2.71	29.0	21.5
	138/95	287	139	2.85	3.35	35.0	29.3
	155/104	413	191	3.05	3.69	45.9	36.7
	170/115	569	260	3.10	4.11	58.1	45.2
	177/124	725	345	3.40	4.75	74.7	55.6
	189/132	956	458	3.70	5.73	89.3	69.4
	206/139	1310	613	4.10	6.44	115	88.2
	224/150	1775	830	4.50	7.32	143	111
	237/162	2360	1120	4.85	8.76	176	138
Delta Sect.	274/185	3650	1650	4.90	10.32	232	178
	121/92	205	122	3.00	3.15	28.9	26.5
	129/100	292	175	3.50	3.74	38.9	35.0
	137/113	375	250	3.90	4.21	50.0	44.2
	146/112	508	310	4.40	5.05	61.9	55.3
	160/132	750	500	5.30	6.67	80.6	75.7
Furl. Sect.	190/94	580	200	3.00	4.69	55.4	42.5
	213/104	850	290	3.15	5.45	73.2	55.7
	235/116	1240	435	3.40	6.55	97.6	75.0
	232/126	1590	605	5.00	8.71	128	96
	260/136	2400	900	5.75	10.36	176	132
	290/150	3520	1300	6.00	12.63	224	173
Boom Sect.	86/59	60	23	1.80	1.67	14.0	7.8
	120/62	155	42	1.80	2.16	24.8	13.7
	143/76	290	80	2.20	2.83	39.4	20.9
	162/125	615	330	2.80	4.75	76.0	53.0
	171/94	610	170	2.80	4.03	67.7	35.7
	200/117	1190	325	2.80	5.36	112	55.5
Spinn Pole Sect.	250/140	2410	640	3.20	6.96	185	91.4
	48/48	7.65	7.65	2.00	0.75		
	60/60	15.4	15.4	2.00	1.00		
	72/72	29.9	29.9	2.20	1.38		
	84/84	48.8	48.8	2.20	1.53		
	96/96	72.3	72.3	2.20	1.76		
	99/99	123	123	3.60	2.65		
	111/111	197	197	4.10	3.38		

Matching components made of stainless steel, type AISI-316

1x19 Wire			Rigging Screw		Chainplate lug (see fig.)			
Diam (mm)	Br.str (N)	Weight (kg/m)	Diam (in)	Br.str (N)	a (mm)	b (mm)	c (mm)	d (mm)
3	7700	0.040	1/4	14700	20.0	5.0	12.0	8.5
4	13800	0.073	5/16	22600	22.0	6.0	13.0	10.0
5	21600	0.113	3/8	33400	25.0	8.0	16.0	12.0
5.5	25700	0.139	7/16	46100	30.0	10.0	18.0	14.0
6	30000	0.165	7/16	46100	36.0	10.0	21.0	14.0
7	40900	0.225	1/2	66700	38.0	12.0	24.0	16.0
8	53500	0.327	5/8	93200	40.0	13.0	25.0	16.0
10	69100	0.475	3/4	123000	45.0	14.0	27.0	18.0
11	83500	0.648	3/4	123000	50.0	14.0	30.0	18.0
12	120200	0.820	7/8	167000	60.0	18.0	36.0	22.0
14	160100	1.000	1	218000	65.0	22.0	38.0	25.0



Chainplate lug
a = width
b = thickness
c = centre of hole to top of lug
d = diameter of hole

